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TOMAND A GENERAL THEORY OF FLATES AND SHELLS
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## TOWARD A GENERAL THEORY OF PLATES AND SHELLS WITE VINITE DISPLACEMENTS AND STRAIRS

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In the first two sections of the present paper a short presentation of the nonlinear theory of shells is given using the usual saysmetric tensors of the tangential forces and agments  $T_{\bullet}^{\alpha\beta}$  and  $H^{\alpha\beta}$ . For small strains in a thin shell these tensors can be considered to be symmetrical to the accuracy of the Kirchhof hypothesis; i.e.,  $T_* \sim T_* \sim T_$ a shell of medium thickness, this simplification leads to an error of the order /b/R compared to unity, and to an even higher error for shells made of material not obeying a linear Hooke's law. If the force and moment tensors are not symmetrical, excessive difficulties arise in formulating the nonlinear relations of elasticity, the general theorems of the nonlinear theory of shells, the evaluation of the accuracy of apprimation methods, etc. Therefore in Section 3 of this paper, the basic force and moment tensors are introduced in a form symmetric for arbitrary strains. In this case it is quite natural to assume a distribution of tangential stresses over the thickness of the shell rather than to make geometrical assumptions. Using the symmetrical force and expent tensors it is possible to find a general expression for the strain potential and to express the general integrals by homogeneous equilibrium equations, not through four functions as is usually done, but by three.

In Section 4 the general equations of clasticity for isotropic shells are derived.

In Section 5 it is proved that the Galerkin equations in the theory of finite strains is not directly connected with the principle of minimum potential energy as is the case in the linear theory.

In Section 5 a functional R is introduced which has a stationary value when static values of the boundary conditions and the equations of equilibrium are reached. For the simplified equations the corresponding functional is considered in Reference 1. Since the Kirchhof hypothesis is not used to obtain equation (6.22) and the strains are considered to be arbitrary, the variational equation SR = 0 is applicable to a shell of medium thickness as well as to physically nonlinear problems.

In the functional of (6.22), in addition to the forces and moments, the quantities  $e_{\alpha\beta}$  and  $\omega_{\alpha}$ ; these satisfy three geometrical identities, which do not appear as simultaneous conditions for finite strains. Therefore an adjacent stress condition is not required to satisfy the conditions of continuity of finite strains and the variational principle SR = 0 is analogous in a physical sense to the Castigliano principle in the linear theory of elasticity.

In Section 7 the functional R is transformed to the form (7.11), which does not contain a displacement.

1. First and Second Strain Temsors of a Surface. We refer the median surface 8 of an unstrained shell to Caussian coordinates  $x^2$  and  $x^2$  and introduce the following notations:  $\beta$  is the radius vector of the point  $(x^2, x^2)$ ,  $\beta$  are the coordinate vectors of the surface  $\beta$ ,  $\alpha$  is the unit vector normal to  $\beta$  at the point  $(x^2, x^2)$ ,  $\alpha$  and  $\beta$  are the components of the first and second metric tensors of the surface S, so that

o d are the components of the discriminant tensor:

For the right-hand oriented trihedron  $\rho_1 \rho_{2m}$  we have

$$P_{\alpha} \times P_{\beta} = c_{\alpha\beta} \text{ m, m x } P_{\alpha} = c_{\alpha\beta} P^{\beta} \tag{1.1}$$

where  $P_{\beta} = a \propto P_{\alpha}$  are the vectors of the reciprocal basis ad sean BX

For the vectors / and n the following equations are valid:

$$\nabla_{\alpha} \rho_{\beta} = b_{\alpha\beta} \pi, \nabla_{\alpha} \rho^{\beta} = m \rho^{\beta}, \pi_{\alpha} = -b \rho^{\beta} \rho_{\beta} \qquad (1.2)$$

Here and in the following, V is a sign for coverient differentiation with respect to a ~ B.

Let G be the contour of the median surface, s its arc length, n and  $\mathcal{C}$  unit vectors normal and tengent to C. The components of n and  $\mathcal{C}$  in the system of coordinates  $x^2$  and  $x^2$  on S are given by the formulas

$$n_{\lambda} ds = c_{\lambda\beta} dx^{\beta}, \quad \zeta^{\alpha} ds = dx^{\alpha}, \qquad (1.3)$$

$$n^{\alpha} = c^{\alpha\beta} C_{\beta}, \quad C_{\beta} = c^{\alpha\beta} n_{\alpha}$$

In the following, all geometrical and physical quantities referring to the strained surface S\* vill be starred. The formulas given above hold also for the strained surface if f,  $f_{\infty}$ , n,  $n_{\infty,0}$ ,  $n_$ 

Let v be the displacement vector of the point  $(x^1, x^2)$  of the surface s, /\* = / + v is the radius vector of this point after strain deformation, i.e., of the point  $(x^1, x^2)$  of the surface  $s^*$ . The components of the first tensor (p) and the second tensor (q) of strain are determined by the equations

$$2 \alpha \beta = \alpha \beta = \alpha \beta = \alpha \beta + e_{p\alpha} + a^{R\lambda} e_{n} e_{p\lambda} + \omega_{\alpha} \omega_{\beta}, \quad (1.4)$$

$$2 \alpha \beta = 0 \alpha \beta = 0 \alpha \beta$$

where

$$e_{\perp\beta} = \nabla_{\alpha} v_{\beta} - v_{\alpha\beta}, \quad \omega_{\alpha} = \nabla_{\alpha} \omega + b_{\alpha}^{\prime} v_{\lambda}, \quad (1.6)$$

$$28 = e^{\alpha \beta} c_{\pi \lambda} (S_{\alpha}^{\pi} + e_{\lambda}^{\pi}) (S_{\beta}^{\lambda} + e_{\beta}^{\lambda}), \qquad (2.6)$$

For the coefficients of the connectivity  $\Gamma_{\alpha\beta}^{\gamma}$  and  $\Gamma_{\alpha\beta}^{*\gamma}$  of the surfaces S and S\* we have the relations

$$\Gamma_{\alpha\beta}^{*} = \Gamma_{\alpha\beta}^{\vee} + a_{\alpha}^{\vee} \lambda_{\beta}^{P} \lambda_{\alpha\beta} \quad (P \lambda_{\alpha\beta} = \nabla_{\alpha} P_{\beta\lambda}^{P} + \nabla_{\alpha} P_{\alpha\beta}^{P})$$

$$\nabla_{\beta} P_{\alpha\lambda}^{P} - \nabla_{\lambda} P_{\alpha\beta}^{P}$$
(1.10)

The tensor  $P_{\text{off}}$  depends on the angles of rotation of an element of the median surface. In fact, putting  $V_{\text{off}} P_{\text{off}} = b_{\text{off}} p_{\text{off}}$  (where  $V_{\text{off}}$  is a sign of coveriant differentiation with respect to  $a_{\text{off}}$  in the expression

we have

$$P_{\gamma,\alpha\beta} = P_{\gamma} * \cdot \nabla_{\alpha} P_{\beta} * = P_{\gamma} * (\nabla_{\alpha} P_{\beta} + \nabla_{\alpha} \nabla_{\beta} *) = (P_{\gamma} + P_{\gamma} * P_{\gamma} + m \omega_{\gamma}) \cdot (mb_{\alpha\beta} + \nabla_{\alpha} \nabla_{\beta} *)$$

Bence, we find

$$P_{\gamma,\alpha\beta} = \omega_{\gamma} (b_{\alpha\beta} + b_{\alpha}) + \Omega_{\alpha\beta}^{2} (a_{\gamma\lambda} + a_{\gamma\lambda})$$
 (1.11)

2. The Fountions of Equilibrium. Let the shell be in a state of equilibrium under the action of the given forces. We subject it to an infinitesimally small possible perturbation characterised by the vector S u and consistent with the constraints present on the shell. Then the initially possible perturbations for a three-dimensional body are expressible by the relations

SA = 
$$\iiint_{\Omega} F_{*} \cdot Suc_{\Omega} + \iiint_{D} F_{*} \cdot Suc_{\Omega} = \iiint_{\Omega} \sigma^{ik} Se_{ik} a_{\Omega} = (2.1)$$

$$\iiint_{\Omega} \sigma^{ik} F_{ik} + \int_{i} F_{*} \cdot \frac{\partial Su}{\partial x^{k}} a_{\Omega}$$

Here  $F^*$  is the force referred to a unit volume  $\Omega$  of the strained body, p is the surface force occurring on a unit strained surface  $\Pi$ , and the  $\mathcal{E}_{1k}$  are components of the tensor of the finite strains.

Applying (2.1) to the strained shell, we get

Here  $T_* \propto \beta$  and  $K = \beta$  are the components of the tensor of the tensor of the tensor that the tensor of the tensor that the tensor of the

$$T_{*}^{\alpha\beta} = \int_{-h(-)}^{h(f)} \sqrt{\frac{g}{g}} e^{\alpha\lambda} \left( \delta_{\lambda}^{\beta} - \sin^{\beta}_{\lambda} \right) dx, \quad \pi^{\alpha} = \int_{-h(-)}^{h(f)} \sqrt{\frac{g}{g}} e^{\alpha\beta} dx$$

$$(2.3)$$

$$H^{\alpha\beta} = \int_{-h(-)}^{h(\beta)} \int_{-h(-)}^{\infty} \sigma^{\alpha\lambda} (\delta_{\lambda}^{\beta} - x^{-\alpha}_{\lambda}^{\beta}) x \, dx$$

Further, s is a coordinate normal to S\*; h( $\varphi$ ), h( $\varphi$ ) are the equations of the boundary surfaces; X\*, M\* are the vectors of the external forces and noments with components X\* $\varphi$  $^{\times}$  and M\* $\varphi$  $^{\times}$  in the coordinate system of the strained surface; X\* $\varphi$  $^{\otimes}$  \* X\* $\varphi$ \*\* are referred to a unit area of the strained surface  $\varphi$ \*;  $\varphi$ \* is the vector of the contour of the load referred to a unit length of the strained contour C\* $\varphi$ ; G\* is the bending moment on this contour.

From the relations (2.2) we get the equations of equilibrium

$$\nabla_{\alpha} = 0, \quad \nabla_{\alpha} = 0, \quad \nabla_{$$

or in vestor form

$$\nabla_{x} * x_{*} < \neq x_{*} = 0, \quad \nabla_{x} * x_{*} < \neq \beta_{x} * x_{*} < \neq \lambda_{k} = 0$$
 (2.5)

end the otatic boundary conditions

$$\Phi_{+} = P - \frac{\partial \mathbf{n}_{+}^{\beta}}{\partial s}, \quad \mathbf{n}_{+} = \mathbf{E}^{\alpha\beta} \mathbf{n}_{\lambda} + \mathbf{n}_{\beta}$$
 (2.5)

Where

Putting  $\Phi^* = \Phi_* \sim \rho_{\alpha} + \Phi_*^{3} n_{\bullet}$ , from (2.5) we obtain the scalar form

$$\Phi_* = T_* \beta \alpha_{n_{\beta}} + \beta * \beta T_{\beta} * n_{\beta} \qquad \Phi_3 * = R_* \alpha_{n_{\gamma}} + \frac{2\pi}{3}$$
 (2.9)

If stretching and shear are negligible with respect to unity, the relation (2.2) can be written in the form

where

can be taken as the second strain tensor3, 4.

Let T  $\prec^{\beta}$  be the tangential stresses in pure strain axes, and  $\varepsilon_{\prec\beta}$  the components of the pure strain:

$$20_{AB} = 2 \epsilon_{AB} + \epsilon_{A}^{Y} \epsilon_{YB} \qquad (2.12)$$

Then putting Sp from (2.12) into (2.2) we find SW, the increment in the strain energy of the shell referred to a unit area of the unstrained median surface:

where

As a result, for small strains  $T^{\prec\beta} \approx T_{\ast}^{\prec\beta}$ ,  $F_{\ast} \approx \mathcal{E}_{\prec\beta}$ . A theory of shells, in which the magnitudes are related to the exes "after rotation" (to the exes of pure strain) is built up in the vork of E. A. Alusyse.

The basic equations of the nonlinear theory of shells in Euler variables and their transformation to Lagrange variables are given in articles by Synge and Chien<sup>5</sup>. However the accuracy of the equations obtained is greater than the accuracy of a linear Roche's law. Therefore the theory given in these works because comprehensible only as a result of great simplifications. In the derivation of the equilibrium equations (2.4), the coordinate system is made up of Gaussian coordinates x<sup>1</sup> and x<sup>2</sup> on 3\* and a third coordinate x, which is the distance of a point on the deformed shell to the surface 8\*.

If use is made of the relations (1.10), the equilibrium equations are transformed to the form

$$\nabla_{\alpha} \left( \sum_{i=1}^{n} T_{\alpha}^{\alpha\beta} \right) \neq \sum_{i=1}^{n} \left( n_{\alpha}^{\beta} \lambda_{i} \right) + \sum_{i=1}^{n} \left( n_{\alpha}^{\beta} \lambda_$$

x\* (3) = 0

$$\nabla_{\alpha} \left( \sum_{k=1}^{\infty} \mathbf{x}_{k}^{\alpha} \right) \neq \sum_{k=1}^{\infty} \left( \mathbf{b}_{\alpha/\beta} + \mathbf{x}_{k}^{\alpha/\beta} + \mathbf{x}_{k}^{\alpha/\beta} \right) = 0 \tag{2.14}$$

$$\nabla_{\mathcal{A}} \left( \sum_{\alpha} \mathbf{H}^{\alpha} \beta \right) \neq \sum_{\alpha} \left( \mathbf{a}_{\alpha} \beta \lambda \mathbf{P}_{\lambda_{\alpha} \propto \gamma} \mathbf{H}^{\alpha \gamma} - \mathbf{H}_{\alpha} \beta \right)$$
 (2.15)

and in the case of small strains (a,  $\alpha$  a, a,  $\alpha^{\beta} \approx a^{\beta}$ ) to the form

$$\nabla_{\alpha} T_{*}^{\alpha\beta} + a^{\beta\lambda} P_{\lambda \alpha \gamma} T_{*}^{\alpha\gamma} - a^{\beta\gamma} (b_{\alpha\gamma} + q_{\alpha\gamma}) R_{*}^{\alpha} + (2.16)$$

$$\nabla_{x} x_{*}^{\alpha} + (b_{\alpha\beta} + q_{\alpha\beta}) x_{*}^{\alpha\beta} + x^{3} = 0$$
 (2.17)

Here  $X^S$ ,  $X^S$ ,  $X^S$  are the components of the external forces and moments in the coordinate system of the deformed shell related to a unit area of the undeformed surface. The boundary conditions (2.9) after substitution of the covariant components of the vectors normal and tangent to the undeformed contour of the shell can be written in the form

$$\Phi_* \stackrel{\text{de}^*}{\text{ds}} = n_{\alpha} T_*^{\alpha\beta} + \tau^{\beta} b_{\beta}^{\alpha} H, \quad \Phi_3 \stackrel{\text{de}^*}{\text{ds}} = n_{\alpha} n_{\alpha}^{\alpha} - \frac{\partial H}{\partial s}$$

$$\int_{a_*}^{a_*} \left(\frac{ds_*}{ds}\right)^2 G_* = H^{\alpha\beta} n_{\alpha} n_{\beta}$$
(2.19)

where n and  $\mathcal{C}_{\alpha}$  are the components normal and tangent to the contour C in a system of coordinates on the surface S, and ds and ds\* are elements of path length of the contour of the shell up to and after deformation:

It should be noted that (2.16), (2.17) and (2.18) contain in an unexplicit form the equilibrium equations of the theory of shells for large displacements obtained by Lyav and other authors. If one retains in them second-order infinitesimals with respect to displacements and their derivatives, i.e., if one puts

then they go over into the equations of Lyav in generalized co-ordinates.

We suppose that  $q \sim \int \mathcal{E}_{p}$ ,  $e \sim p \sim \mathcal{E}_{p}$ , where the symbol  $\sim$  indicates that the quantities despared have the same order of magnitude, and  $\mathcal{E}_{p}$  is the maximum relative extension within the limits of proportionality. The theory of shells based on these assumptions is comparable in accuracy with von Karman's theory of plates. We call this kind of a deflection of a shell a mean deflection. In this case the following approximate formulas are correct:

Substituting q and P from here into (2.16), (2.17) and (2.18) we get an equation for the mean deflection; referred to the lines of curvature they agree with the equilibrium equations of Kh. M. Mushtari. These refined equations are necessary in problems of stability and in impact phenomena.

If the external forces and moments do not depend on the strains, the problems in the theory of shells can be solved either in terms of forces and moments or in components of the strains, by adding to the system (2.16), (2.17), and (2.18) the common strain conditions.

The equilibrium equations for the theory of small strains (2.16), (2.17) and (2.18) can be simplified. Up to the loss of stability, terms containing small multiples of  $P_{\text{con}} > \rho$  can be neglected since they are quantities of the order  $h \in \rho^2$  and in the presence of edge effects the ratio of these terms to the main ones will be of the order  $g_{\text{con}}$ . The neglect of these quadratic terms amounts to a significant limitation only when terms of the equations, which we have assumed to be the principal ones, themselves are small on account of the natural cancellation of their principal terms (for instance, in the solution of the problem of the stability of very long cylindrical tubes during small compression.

3. Introduction of Symmetrical Tensors of Force and Momentus. We introduce symmetric tensors of the tangential forces and momenta, putting

where

Substituting Ta do in the sixth equilibrium equation

we get  $c_{\alpha\beta} = \alpha^{\beta} = 0$ , i.e.,  $s_*^{\alpha\beta}$  is a symmetric tensor.

An antisymmetric tensor  $Q^{\alpha\beta} = -Q^{\beta\alpha}$  can be determined from the supplementary nondifferential condition which assures matching of the tangential tensions at all points through the thickness of the shell.

On retaining the term of anallest degree in h this relation will be

Futting (3.1) in (2.2) and considering that the scalar product of a symmetric tensor by an antisymmetric one is equal to zero, we obtain

$$SA = \iint_{\mathcal{A}} (\mathbf{x}, \alpha)^{\beta} S_{\alpha\beta} - \mathbf{x}^{\alpha\beta} S_{\alpha\beta} )d\sigma^{*} =$$

$$\iint_{\mathcal{A}} \{(\mathbf{x}, \alpha)^{\beta} - \mathbf{x}^{\beta} \mathbf{x}^{\gamma\alpha}) \rho_{\beta}^{*} \cdot S\rho_{\alpha}^{*} +$$

$$\lim_{\alpha \to \beta} \rho_{\beta}^{*} \cdot \frac{\partial S_{\alpha\beta}}{\partial \mathbf{x}^{\gamma}} \}d\sigma^{*}$$

$$(3.4)$$

From this the equilibrium equations follow\*

Whote: It follows from (3.5) that the simplification  $T_* \curvearrowright \nearrow T_* \nearrow \nearrow$  in the case of a thin shell is only permissible when the shear forces  $H_* \curvearrowright$  of the first two equilibrium equations are negligible.

$$\nabla_{\chi} * (\bullet_{+} \times \beta - \bullet_{\gamma} ) * * \gamma ) - \bullet_{\gamma} \circ_{*} \times \not + X_{*} \beta = 0$$
 (3.5)

$$\nabla_{x} = \frac{1}{2} + \frac{1}{2$$

and the static boundary conditions

$$\Phi_{\bullet} = (\bullet_{\bullet} = \bullet_{\uparrow} = \bullet_$$

where  $B_1 = -M_p^{\alpha\beta} n_{\alpha} + C_{\beta}^{\alpha}$  is the twisting moment on the contour,  $Q_p^{\alpha}$  is a vector similar to the vector of the shear forces. Thus there are essentially six unknowns  $a_p^{\alpha\beta}$  and  $M_p^{\alpha\beta}$  in the theory of shells instead of eight. The same observations were made by A. I. Lur'ye. Since for an isothermal deformation process the work of deformation is a total differential, from (3.4) we have the general relations of elasticity

$$\sqrt{\frac{\partial \mathbf{r}}{\partial \mathbf{r}}} = \frac{\partial \mathbf{r}}{\partial \mathbf{r}_{\alpha\beta}} \sqrt{\frac{\mathbf{r}}{\mathbf{r}}} = \frac{\partial \mathbf{r}}{\partial \mathbf{r}_{\alpha\beta}}$$
(3.9)

where W is the energy of deformation of the shell, related to a unit area of the mean undeformed surface.

We introduce still another tensor of forces and moments, putting

$$\int_{\mathbb{R}} Q_{\alpha}^{\alpha} = \frac{\partial V}{\partial P_{\alpha} \beta}, \quad \int_{\mathbb{R}} \frac{\partial V}{\partial P_{\alpha} \beta} = \frac{\partial V}{\partial Q_{\alpha} \beta}$$
 (3.10)

as well as vectors of the external forces and moments, related to a unit surface of the mean undeformed surface:

$$X = \sqrt{\frac{8}{2}} X_{*}, M = \sqrt{\frac{8}{2}} M_{*}$$
 (3.11)

Then from (3.5)-(3.7) we get the equations of equilibrium in the form

$$\nabla_{\alpha} (\mathbf{s}^{\alpha\beta} - \mathbf{m}_{\gamma}^{\beta} \mathbf{N}^{\gamma\alpha}) / \mathbf{s}_{\alpha}^{\beta\gamma} P_{\lambda,\alpha\sigma} (\mathbf{s}^{\gamma\sigma} - \mathbf{m}_{\gamma}^{\sigma} \mathbf{N}^{\gamma\sigma}) - (3.12)$$

$$\mathbf{m}_{\alpha}^{\beta} \mathbf{q}^{\alpha} / \mathbf{X}^{\beta} = 0$$

$$\nabla_{\alpha} Q^{\alpha} \neq b =_{\alpha \beta} (b^{\alpha \beta} - b_{\gamma} \beta N^{\gamma \alpha}) \neq x^{3} = 0$$
 (3.13)

$$\nabla_{\alpha} \mathbf{N}^{\alpha\beta} / \mathbf{e}_{\alpha}^{\beta\lambda} \mathbf{P}_{\lambda,\alpha\gamma} \mathbf{N}^{\alpha\gamma} - \mathbf{Q}^{\beta} - \mathbf{M}^{\alpha} \mathbf{e}_{\alpha\gamma}^{\gamma\beta} \mathbf{e}_{\alpha\gamma} * = 0$$
 (3.14)

and from (3.8) the static boundary conditions in the form

$$\Phi^{\alpha} = (s^{\alpha\beta} - b^*)^{N} (s^{\alpha})_{n_{\beta}} + b^* (s^{\alpha})_{n_{\beta}} + b^* (s^{\alpha})_{n_{\beta}} = c^{\alpha} (s^{\alpha})_{n_{\beta}} - \frac{2\pi}{2\pi}$$
(3.15)

$$\sqrt{\frac{1}{2}} \frac{ds}{ds} G = H^{\alpha\beta} n_{\alpha} n_{\beta} , \left(\frac{ds}{ds}\right)^{2} R_{2} = -H^{\alpha\beta} n_{\alpha} \tau_{\beta}$$

In these equations  $\nabla$  is the sign of covariant differentiation with respect to  $\mathbf{x}_{\infty}$ ;  $\mathbf{X}^{\infty}$ ,  $\mathbf{H}^{\infty}$  are the contravariant components of the vectors  $\mathbf{X}$  and  $\mathbf{H}$  in the coordinate system of the mean deformed surface;  $\mathbf{X}^{3}$  s<sub>m</sub> $\mathbf{X}$ ;  $\Phi^{\infty}$  and  $\Phi_{3}$  are the components of the contour force related to a unit length of the undeformed contour  $\mathbf{C}$  of the shell in the same system of coordinates;  $\mathbf{G}$  is the bending moment related to a unit length of the undeformed contour. It is important to note that for usual displacements and strains the first and second strain tensors admit of a potential. In fact, introducing the function  $\mathbf{F}$  defined by

$$F = \mathbf{R}^{\alpha\beta} \mathbf{p}_{\alpha\beta} - \mathbf{M}^{\alpha\beta} \mathbf{q}_{\alpha\beta} - \mathbf{W} \tag{3.16}$$

from the relation

taking (3.10) into account we obtain formulas similar to the Castigliano formulas in the linear theory:

$$P_{\alpha\beta} = \frac{\partial V}{\partial * \alpha\beta}, P_{\alpha\beta} = -\frac{\partial V}{\partial M \alpha\beta}$$
 (3.17)

In the case of small displacements and shears we have

$$2F = s^{\alpha\beta} p_{\alpha\beta} - H^{\alpha\beta} q_{\alpha\beta}$$
 (3.18)

In this case for an elastic condition of the shell, F is the work of deformation, and for an elastic-plastic condition it is the additional work. We present equations (3.5)-(3.7) in the vectorial form

$$\nabla_{x} = e_{1} \times f_{x_{+}} = 0, \quad \nabla_{x} = e_{x_{+}} \times f_{x_{+}} \times f_{x_{+}} \times f_{x_{+}} = 0$$
 (3.19)

**vbere** 

then the equations are satisfied for X\* = M\* = O, putting

$$r_1^{\alpha} = c_* \alpha \beta \nabla_{\beta} * \varphi, \quad L_{\alpha}^{\alpha} = c_* \alpha \beta \nabla_{\beta} * \psi + (\rho_{\beta} * * \varphi) J \qquad (3.21)$$

where  $\varphi$  and  $\psi$  are certain vectors. Since  $L_b \cong 0$ , these vectors satisfy the conditions

$$\varphi^{\alpha} = e_{\bullet}^{\alpha\beta} (\nabla_{\beta} \varphi + \varphi^{\lambda} b_{\lambda\beta}^{\bullet}) \qquad (3.22)$$

and

$$\varphi = \varphi^{\alpha} \rho_{\alpha} * / m_{\bullet} \varphi, \quad \psi = \psi^{\alpha} \rho_{\alpha} * / m_{\bullet} \psi \qquad (3.23)$$

From a comparison of (3.20) and (3.21) we get

$$\mathbf{e}_{*} \propto \beta = \mathbf{e}_{*} \propto \pi_{\mathbf{c}_{*}} \beta_{\gamma} \nabla_{\pi} * (\nabla_{\gamma} \psi + b * \gamma \psi) - \tag{3.24}$$

$$N_{\bullet}^{A\beta} : c_{\bullet}^{A} \wedge R c_{\bullet}^{A} \wedge P \wedge (\nabla_{\mathcal{R}}^{*} \psi_{Y} - b_{\mathcal{R}Y}^{*} \psi) - c_{\bullet}^{A\beta} \phi \qquad (3.25)$$

Since  $\psi_{\alpha} = 0$ , it follows from (3.25) that  $2 \varphi = e_{\alpha} \times \varphi_{\alpha} = 0$ . Thus the general solution of the homogeneous system contains three independent functions  $\varphi$  and  $\varphi_{\alpha}$ .

h. Lass of Electicity and Strengthening. Let  $\mathcal{E}_{1k}$  be the covariant components of the finite strain tensor ( $\mathcal{E}_{1k}^{+}\mathcal{E}_{1k}^{-18}\mathcal{E}_{k8}$ ),  $\sigma^{1k}$  the contravariant components of the stress tensor in the coordinate system of the shell after deformation; and  $V^0$  the energy density for deformation of the shell considered as a three-dimensional body. Then for an isotropic shell during deformations of usual magnitude we have the nonlinear relations

$$\sigma^{4k}\sqrt{s} = s^{4k}(\frac{\partial V^0}{\partial s_1} \neq s_1 \frac{\partial V^0}{\partial s_2} \neq s_2 \frac{\partial V^0}{\partial s_3}) - (\frac{\partial V^0}{\partial s_2} \neq s_1 \frac{\partial V^0}{\partial s_3}) \in {}^{4k} \neq$$

where  $x^0$  depends only on the invertents of the tensor  $\epsilon_k^i$ .

$$s_1 = s^{ik} \epsilon_{ik}$$
,  $as_2 = s_1^2 - \epsilon_k^1 \epsilon_1^k$ ,  $s_3 = \det(\epsilon_k^1)$ , (4.1)

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If we take the Kirchhof hypothesis ( $\epsilon_3^{1} = \epsilon_3^{2} = 0$ ), then the relations of (4.1) are considerably simplified, since for this case

$$s_1 = o_1 + e_3^3$$
,  $s_2 = o_1 e_3^3 + o_2$ ,  $s_3 = o_2 e_3^3$  (4.2)

where the quantities  $\Theta_1$  and  $\Theta_2$  are expressed by the formulas

and are the invariants with respect to a transformation of Gaussian coordinates on the surfaces  $x^3$  = const. To determine  $\epsilon_3$  3 we take as usual  $\sigma^{33}$  = 0. Then putting in (4.1) i = k = 3 and introducing in the relations obtained  $s_1$ ,  $s_2$ , and  $s_3$  from (4.2), we obtain an equation of the form

$$\mathfrak{L}(\Theta_1, \Theta_2, \varepsilon_3^2) = 0 (4.4)$$

We note further that in the relations (4.1) for i, k = 1, 2 the expressions

Then substituting this last expression in (4.1) and taking into account (4.2) and (4.4) we get the condition for the elasticity of an isotropic shell

$$\sigma^{\alpha\beta} = A_1^{\alpha\beta} + B_1^{\alpha\beta} R^{\alpha\beta} \epsilon_{\pi\lambda} (\alpha, \beta, \pi, \lambda = 1.2)$$
 (4.5)

where  $A_1 \stackrel{\mathcal{C}}{\sim} and B_1 \stackrel{\mathcal{C}}{\sim} \mathcal{C}^{\mathcal{R} \mathcal{A}}$  are functions of the invariants  $\Theta_1$  and  $\Theta_2$  only.

In order to express the forces and moments by means of the deformations of the surface it is necessary to consider that  $\Theta_1$  and  $\Theta_2$  depend on the following invariants of the strain tensors of the surface:

$$p_1 = a^{\alpha\beta} p_{\alpha\beta}$$
,  $q_1 = a^{\alpha\beta} q_{\alpha\beta}$ ,  $p_2 = det(p_{\gamma}^{\alpha})$ ,  $q_2 = det(q_{\gamma}^{\alpha})$  (4.6)  
 $p_1 = b^{\alpha\beta} p_{\alpha\beta}$ ,  $q_1 = b^{\alpha\beta} q_{\alpha\beta}$ ,  $p_2 = det(p_{\gamma}^{\alpha})$ ,  $q_2 = det(q_{\gamma}^{\alpha})$  (4.6)

Then W, the potential of the forces and moments, will depend in the general case on those eight parameters.

Therefore, we note that

$$\frac{\partial P_1}{\partial P_{\alpha\beta}} = \alpha^{\beta} \cdot \frac{\partial P_1}{\partial P_{\alpha\beta}} = \alpha^{\beta} \cdot \frac{\partial P_2}{\partial P_{\alpha\beta}} = \frac{\partial P_1}{\partial P_{\alpha\beta}} = \alpha^{\beta} \cdot \frac{\partial P_2}{\partial P_2} = \alpha^{\beta} \cdot \frac{\partial P_2}$$

From (3.10) we obtain the condition of elasticity

where

$$D_{1} \leftarrow P = \alpha \leftarrow P(\frac{\partial V}{\partial \mathbf{p_{1}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{p_{2}}} + \mathbf{q_{1}} \frac{\partial V}{\partial \mathbf{p_{1}}}) + D \propto P(\frac{\partial V}{\partial \mathbf{p_{1}}} + \mathbf{q_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p_{1}} \frac{\partial V}{\partial \mathbf{q_{2}}}) + D \sim P(\frac{\partial V}{\partial \mathbf{q_{2}}} + \mathbf{p$$

Relation (4.8) is applicable to deformations of usual magnitude and is independent of the Kirchhof hypothesis for conditions such that

We consider the case of small elastic-plastic deformations of a shell with large displacements. In this case to the limits of accuracy of the Kirchhof hypothesis the formulas of the first approximation of Kirchhof-Lyaw may be used. The work of volume deformations in the plastic state is

$$W_0 = x(\frac{2\pi}{\lambda / 2\mu})^2 \left( \ln x^2 / \frac{\hbar^3}{3} q^2 \right) \tag{4.10}$$

where 2h is the thickness of the shell,  $\lambda$ ,  $\mu$  are lame constants, and K is the bulk compression modulus. Since the energy density for change of form in the plastic state of a material depends only on the second invariant of the deviation of the deformation, the invariants  $p_2$ ,  $q_2$ , r,  $r^*$  enter the expression V through the invariants

$$P_p = (p_1)^2 - p_2$$
,  $P_q = (q_1)^2 - q_2$ ,  $P_{pq} = r' + \frac{1}{2}r$  (4.11)

In view of this and of the relations (4.7) and (4.10) we have

where

$$\begin{array}{lll}
\mathbf{A} \propto \beta \pi \lambda & = \alpha \beta \cdot \pi \lambda & \beta \cdot \lambda & \beta \cdot \lambda & \beta \cdot \lambda & \beta \cdot \lambda \\
\mathbf{B} \propto \beta \pi \lambda & = -\frac{2}{3} \mathbf{I}_{2} \left( \alpha \propto \beta \cdot \pi \lambda + \alpha \times \pi \cdot \lambda + \alpha \times \pi \cdot \lambda \right) \\
\mathbf{C} \propto \beta \pi \lambda & = \left[ \frac{2\pi}{3} \times \left( \frac{2\pi}{3} \times \frac{2\pi}{3} \right)^{2} + \frac{2}{3} \mathbf{I}_{3} \right] \alpha \wedge \beta \cdot \pi \lambda + \frac{2}{3} \mathbf{I}_{3} \propto \pi \cdot \lambda + \frac{2}{3} \times \lambda + \frac{2}{3} \propto \pi \cdot \lambda + \frac{2}{3} \propto$$

where

$$L_1 = \frac{3}{2} \frac{\partial V}{\partial P_p}, L_2 = -\frac{3}{2} \frac{\partial V}{\partial P_{qq}}, L_3 = \frac{3}{2} \frac{\partial V}{\partial P_{q}}$$
 (4.14)

are integrals introduced by A. A. Ilyushin 10 in the case of plates.

5. The Learning-Galerkin Equations. Finding an exact solution of the equations of the theory of shalls during large displacements is difficult even in the simplest cases. Therefore, application is made of approximate methods based on variational theorems. In problems of the stability of elastic equilibrium and the investigation of impulse phenomens in shalls, the variational equation (2.2) is usually used, expressing the origin of the possible displacements

If the external forces permit a potential, then the discovery of functions satisfying equation (5.1) comes down to finding a minimum of the total potential energy (Ritz method). The approximation method of Ritz can only be applied to the case where the external forces are given independently of the deformation or they are of the form of a hydrostatic pressure. If the external forces do not satisfy such conditions the problem of finding the critical loading can be solved by application of the variational equations (5.1) or by the Calerkin method.

We note that while in the linear theory of shells the Ritz equations and the Galerkin equations result from the cause of the possible displacements and are identities, in the nonlinear theory the Galerkin equations have no kind of connection with the energy function. The Galerkin equations in the nonlinear theory of shells are derived not with reference to the cause of the possible displacements (5.1) but from the variational equations:

where

In fact, integrating by parts we obtain from (5.2)  $\int \{(\bar{\Phi} - P + \frac{\partial_{\mu} m_{\mu}}{\partial \sigma})(P *^{\alpha} \delta v_{\alpha} + m_{\mu} \delta v) + (c_{\mu} \times^{\beta} c_{\mu} \tau_{\beta} * - (5.3)) \delta v_{\alpha} + (\nabla_{\alpha} *^{\alpha} *^{\beta} + (\nabla_{\alpha} *^{$ 

Hence, substituting

12 = - Swa - Px \* · Sm

where  $A^{\lambda}$  and  $B^{\lambda}$  are constants, we get the Calerkin equations.

As is known, when the same system of functions  $f_{\infty}$  and  $f_{\infty}$  are chosen the results of the solution of the linear problems by the method of Ritz and Calerkin is identical; the Galerkin method is used to reduce significantly the amount of computational work. This to a large degree is the advantage of the Galerkin method in nonlinear problems also. This can be seen just from a comparison of the variational equations (5.1) and (5.2), from which the Ritz and Galerkin equations follow, and also from a comparison of the relations (5.1) and (5.3). The right-hand side of (5.3) is obtained from the right-hand side of (5.1) by calculating the expression

$$\iint_{C^*} \{ (\nabla_{\alpha} + \nabla_{\alpha} +$$

Together with (5.1) the relation (5.2) can be successfully applied to the solution of problems in the nonlinear theory. For small strains this relation is simplified, since in a number of cases it is possible to neglect differences in covariant differentiation with respect to  $a_{\times\beta}$  and  $a_{\times\beta}$ . In addition, it is possible to neglect the effect of forces of the type  $R_{\bullet} \sim q < cn$  the possible displacements  $\delta v_{\beta}$ . Thus instead of (5.2) we get

The right-hand side of this relation is only slightly different in form from the strain energy calculated according to the linear theory, but the external forces and moments are given in

the coordinate system of the deformed shell. Let  $X^{\infty}$  and  $X^{3}$  be the components of the external forces, and  $B_{1}$  the components of the external momentum in the coordinate system of the undeformed shell. According to the formulas given in our paper  $X^{3}$ :

$$\begin{array}{lll}
\mathbb{E}_{\mathbf{x}} & & \mathbb{E}_{\rho} \left( \mathbf{x}^{\rho} + \mathbf{e}^{\alpha \rho} \right) + \mathbb{E}_{3} \omega^{\alpha}, & \mathbb{E}_{3} & \mathbb{E}_{\alpha} \mathbb{E}_{3} + \mathbb{E}_{\alpha} \mathbb{E}^{\alpha} \\
\mathbb{E}_{\alpha} & & \mathbb{E}_{\alpha} & \mathbb{E}_{\alpha} & \mathbb{E}_{\alpha} & \mathbb{E}_{\beta} & \mathbb{E}_{\alpha} & \mathbb{E}_{\beta} & \mathbb{E}_{\alpha} & \mathbb{E}_{\beta} \\
\mathbb{E}_{\alpha} & & \mathbb{E}_{\alpha} & \mathbb{E}_{\alpha} & \mathbb{E}_{\alpha} & \mathbb{E}_{\beta} & \mathbb{E}_{\alpha} & \mathbb{E}_{\beta} & \mathbb{E}_{\alpha} & \mathbb{E}_{\beta} & \mathbb{E}_{\alpha} & \mathbb{E}_{\beta} & \mathbb{E}_{\alpha} & \mathbb{E}_$$

where  $\Phi_{2}$  and  $\Phi_{3}$  are the components of the contour force in the system of coordinates of the undeformed shell. If one neglects the quantities  $e_{2,0}$  with respect to unity, from (5.5) and (5.6) we have

$$X_*^{\alpha} : X^{\alpha} + X^3 \omega^{\alpha}, X_*^3 : X^3 - X^{\alpha} \omega_{\alpha}, X_*^{\beta} : X_*^{\beta}$$

$$\overline{\Phi}_*^{\alpha} : \overline{\Phi}^{\alpha} + \overline{\Phi}_3 \omega^{\alpha}, \overline{\Phi}_3^3 : \overline{\Phi}^3 - \underline{\Phi}^{\alpha} \omega_{\alpha}$$

In addition, it is sufficiently accurate to put  $q_{\alpha\beta} = \nabla_{\alpha}\omega_{\beta}$ .

6. Variations of the Stressed State of the Shell with Finite Strains. Let  $\mathbf{v}_{\infty}$  be the covariant components of the displacement vector in the coordinate system of the deformed surface,  $\mathbf{v}^*$  be the projection of this vector on the normal to this surface  $\mathbf{v}^*$ , and  $\mathbf{e}_{\infty,\beta}$  and  $\mathbf{e}_{\infty,\beta}$  the components of the displacement dyad in the same coordinate system:

Then for components of the first strain tensor, from

$$\mathcal{L}_{\alpha\beta} = \mathcal{L}_{\alpha} \cdot \mathcal{L}_{\beta} \cdot \mathcal{L}_{\alpha} \cdot \mathcal{L}_{\beta} = \mathcal{L}_{\beta} \cdot (\mathcal{L}_{\alpha} + \frac{\partial \mathbf{v}}{\partial \mathbf{z}}) - \mathcal{L}_{\alpha} \cdot (\mathcal{L}_{\beta} + \frac{\partial \mathbf{v}}{\partial \mathbf{z}})$$

we have the new expressions

Hence, comparing (6.2) with (1.4), we get

$$e_{\alpha\beta} * = e_{\alpha\beta} / e_{\alpha}^{\lambda} e_{\beta\lambda} / \omega_{\alpha} \omega_{\beta}$$
 (6.3)

and from the equations

we get

$$Q_{\alpha\beta} = (m \cdot m \cdot 1)^{b}_{\alpha\beta} + \omega_{\alpha}^{\gamma} \circ_{\beta\gamma} * + \nabla_{\alpha} \omega_{\beta} * = (6.4)$$

$$(m \cdot m \cdot 1)^{b}_{\alpha\beta} + \omega_{\alpha}^{\gamma} \circ_{\beta\gamma} * + \nabla_{\alpha} * \omega_{\beta} * + * \omega_{\alpha}^{\gamma} \circ_{\gamma, \alpha\beta} \omega_{\mu} *$$

We multiply the vector equilibrium equations (2.5) by the displacement vector v and integrate the result over the entire area of the deformed mean surface

or, using the transformation formula

$$\iint_{\sigma^*} \nabla_{a} *(\mathbf{a} \cdot \mathbf{v}) d\sigma^* = \int_{\mathcal{C}^*} \mathbf{a} \cdot \mathbf{v} \, \mathbf{a}_{a} * d\mathbf{s}^* \qquad (6.5)$$

we find

Substituting  $N_0^{\infty}$  here from (2.4) and again using (6.5) we have

$$\iint_{\mathbb{R}^{A}} (\mathbf{x}_{*} \cdot \mathbf{v} + \mathbf{w}_{*}^{B} \mathbf{w}_{A}) d\mathbf{v} = \mathbf{w}_{A} \cdot \mathbf$$

From the expression for the expentum vector Le \*  $(m^* \times P_B^*)$   $\mathbb{R}^{\times} \cap \mathbb{R}^{\times}$  we set

$$\mathbb{R}^{\mathcal{A}_{\mathcal{B}}} = \mathbb{C}_{\mathcal{B}} \omega_{\mathcal{A}} + \mathbb{C}_{\mathcal{B}} \omega_{\mathcal{A}} +$$

Substituting this in (6.6) and taking the function Hv\* to be single valued, we find

$$\iint_{\mathcal{C}} X_{\bullet} \cdot \nabla f = \int_{\mathcal{C}} \mathcal{C}_{\beta\lambda} \times \omega_{\alpha} \times \partial_{\alpha} \times \int_{\mathcal{C}} \{(P - \frac{\partial P}{\partial A}) \cdot \nabla - (6.8) \times \mathcal{C}_{\beta\lambda} \times \omega_{\alpha} \times \partial_{\alpha} \times \partial$$

or, substituting the symmetric tensors (3.1), (3.10) and (3.11) we get

$$\int_{\Omega} (\mathbf{z} \cdot \mathbf{v} + \mathbf{M}^{\beta} \cdot \mathbf{v}) d\mathbf{v} \cdot \mathbf{p} \cdot \mathbf{v} \cdot \mathbf{$$

Here, as before,  $\overline{\Phi}$  is the vector of the contour forces taken with respect to a unit length of the undeformed contour C; X and M are the forces and moments relative to a unit undeformed surface  $\sigma$ . We transform the expression under the integral sign using the right-hand side of (6.8).

Substituting 
$$e_{\alpha\beta}$$
 and  $\nabla_{\alpha} * \omega_{\beta} * f * v_{\alpha} e_{\beta\gamma} * from (6.3)$ 

and (6.4), we get
$$\int_{\alpha} x \cdot v \int_{\beta} v_{\alpha} dv e_{\beta\lambda} * \omega_{\lambda} * dv \int_{\alpha} \int_{\alpha} v \cdot v \cdot dv \int_{\alpha} v \cdot$$

Now we consider the functional

SA' = 855 
$$\{ F + M \times \beta G \cdot M - 1 \} = \{ F +$$

where P is the deformation potential given by formula (3.16) and

and we examine under what conditions equation (6.11) holds.

For this purpose we take it in the form

$$SA^* = \begin{cases} \begin{cases} \{e_{AB} * Se^{AB} - (\nabla_{A} * \omega_{B} *) \\ e_{AB} * \} SA^{AB} \end{cases} \text{ (6.13)}$$

$$D*^{Y}_{AB} e_{BY} *) SA^{AB}_{AB} \begin{cases} e_{AB} + \int \{e_{AB} * Se^{B}_{B} + \omega_{A} S\omega_{B} \} \} d\sigma$$

Here the second integral is equal to:

since

Now on the basis of the equilibrium equations (3.12), (3.13), and (3.14), on integrating by parts using (6.5) we get

The first integral in (6.13) agrees in form with the right-hand side of (6.8). Therefore on integrating by parts from  $I_1$  we get the equilibrium equations and the boundary conditions with the variation of forces and momenta, when the coefficients of the first and second quadratic forms of the deformed surface are constant. Assuming this and substituting (6.15) in (6.13), we finally get

$$\int_{\mathbf{C}} \left\{ \mathbf{v}_{\alpha} + \mathcal{S}(\bar{\Phi}^{\alpha} - \mathbf{r}_{1}^{\alpha\beta} \mathbf{n}_{\beta} - \mathbf{r}_{1}^{\alpha\beta} \mathbf{r}_{\beta} + \mathbf{r}_{2}^{\alpha\beta} \mathbf{r}_{2} \right\} / \mathbf{r} \mathcal{S}(\bar{\Phi}_{3} - \mathbf{q}^{\alpha} \mathbf{n}_{\alpha}) / \mathbf{r} \mathcal{S}(\bar{\Phi}_{3} - \mathbf{q}^{\alpha} \mathbf{n}_{\alpha$$

$$\frac{\partial \mathbf{L}}{\partial \mathbf{L}} = \omega_{\alpha} + \alpha \delta(\mathbf{c} - \sqrt{\mathbf{L}} + \mathbf{L} +$$

vbore

Thus, the variational equation (6.11) is valid if:

## (a) the displacement dyad is varied\*

\* Note: The variations of the components of the dyad are connected by the relations!

(b) the variations of the forces and schemes satisfy the equilibrium equations

$$\nabla_{\alpha} \delta \mathbf{r}_{1}^{\alpha \beta} / \delta(\mathbf{n}^{\beta r} \mathbf{P}_{r,\alpha \lambda} \mathbf{r}_{1}^{\alpha \lambda}) - \delta(\mathbf{n}^{\beta}_{\alpha} \mathbf{q}^{\alpha}) /$$

$$\delta \mathbf{x}^{\beta} = \mathbf{0}$$
(6.17)

$$\nabla_{x} \delta e^{\alpha} / \delta (e_{x} \rho^{-\alpha} 1^{\alpha}) / \delta x^{3} = 0$$
 (6.18)

$$\nabla_{\alpha} \operatorname{SH}^{\alpha\beta} + \operatorname{S(a_{\bullet}}^{\beta\gamma} \operatorname{P}_{\gamma,\alpha} \operatorname{AH}^{\alpha\lambda}) - \operatorname{SQ}^{\beta} - \tag{6.19}$$

$$\operatorname{S(N}^{\alpha} \operatorname{P}_{\alpha}^{\beta\lambda} \operatorname{C}_{\alpha\lambda}) = 0$$

(c) on the contour of the undeformed surface the variations of the forces and moments satisfy the static boundary conditions

$$\frac{\int \bar{\Phi}^{\alpha} = n_{\beta} \int \bar{\Pi}_{1}^{\alpha} \wedge \rho + e^{\beta} \int (m_{\beta} \bar{\Pi}_{2}), \quad \delta \bar{\Phi}_{3} = n_{\alpha} \delta q^{\alpha} - (6.20)}{\frac{\partial \bar{\Pi}_{2}}{\partial s}, \quad \delta c = n_{\alpha} n_{\beta} \delta (\sqrt{\frac{m_{\beta} \bar{\Pi}_{2}}{n_{\beta}}} M^{\alpha} \rho)}$$

In equations (6.17), (6.18) and (6.19) the coefficients  $a_{B}\lambda^{\gamma}P_{\gamma,\alpha\beta}$  and  $b_{\alpha}$  are varied since they depend on the forces and the moments.

In the case of a mean deflection, equation (6.11) is simplified:

$$\iint_{\mathcal{A}} (\mathbf{v} \cdot \delta \mathbf{x} + \omega_{\mathbf{A}} \mathbf{e}^{-\lambda} \mathbf{e}_{\mathbf{A}} \delta \mathbf{M}^{\beta}) \mathbf{e}_{\mathbf{c}} + \int_{\mathbf{c}} (\mathbf{v} \cdot \delta \mathbf{x} - \omega_{\mathbf{A}} \mathbf{e}^{-\lambda} \mathbf{e}^{$$

ciace in this case

We assume that SX = 0, N = 0 and that on the contour of the underformed shell the following condition is satisfied

$$\int_{C} (\mathbf{v} \cdot \delta \mathbf{b} - \omega_{\alpha} + \mathbf{n} \cdot \delta \mathbf{c} - \mathbf{o} \mathbf{n} - \omega_{\alpha} + \delta \mathbf{n} \cdot \delta \mathbf{n}) d\mathbf{s} = 0$$
 (6.22)

Then this theorem holds: the actual stressed state of a shell differs from all statically possible states in that for it the functional

$$E = \iint \left\{ F + M^{\alpha \beta} \left( I_{m} \cdot m^{\alpha} - 1 \right) b_{\alpha \beta} + a_{\alpha}^{\lambda \gamma} o_{\gamma, \alpha \beta} + J + (6.23) \right\}$$

$$\frac{1}{2} B^{\alpha \beta} \left( e_{\alpha \lambda} e_{\beta}^{\lambda} + \omega_{\alpha} \omega_{\beta} \right) \left\{ a \sigma \right\}$$

has a stationary value; i.e., SR = 0.

This theorem also expresses the origin of possible changes of the stressed state in the nonlinear theory of shells.

In the case of infinitely small displacements the stationary value is a minimum, and SR = 0 expresses Castigliano's principle.

The contour condition (6.22) is setisfied if, for example:

(1) the contour is free

$$\Phi^{\sim} \bullet \bullet$$
(6.24)

(2) the contour is rigidly fixed

$$\nabla = \omega_{\alpha} + \alpha^{\alpha} = 0 \quad (\delta_{m} = \delta_{m} = 0) \quad (6.25)$$

- (3) part of the contour is free, and emother part is fixed;
- (4) the contour has a actionless hinged support

(5) the contour is freely supported

$$v = 0, \quad \bar{\phi}^{\alpha} = 0 = 0 \quad (6.27)$$

The equation SR = 0 is the variational formulation of the conditions of continuity for finite deformations of the shall.

In fact, putting

where  $\varphi$  is the vector stress function, the equilibrium equations (3.12) and (3.13) with X = M = 0 also satisfy, as a result of the contour condition (6.22), the equation

end the Ricci identities, we find the conditions for the continuity of finite deformations:

where H and K are the mean and Coussian curvature of the undeferred surface.

We assume that there is no question of a loss of accuracy in the problem considered. Then it is possible to neglect the products  $S^{\times} = \{a_i, and the vector function <math>\psi$  and the two rotation angles  $\omega_{\times}$  are permitted to vary in the functional R. If, accover, the shall is sloped or if v is a rapidly varying function (local loss of stability, edge effect), the terms  $b_{\times}^{\times}v_{\times}$  are small with respect to  $\nabla_{\times}v$  and as a result  $v_{\times}\approx \nabla_{\times}v$ .

In this case the functions  $\varphi$  and v are permitted to vary (of, the work of N. A. Almysye). In the general case, as distinguished from the linear theory, the variation of the stressed state of the shell is accompanied by variations of the three rotation angles:  $\Omega$  =  $\frac{1}{2}$  c $^{\alpha}$ 0  $_{-\beta}$  and  $\omega_{\alpha}$ . This makes it dif-

ficult to moply Contigliano's principle to real problems. But in the case of homogeneous equations the parameters  $e_{\infty}\rho$  and  $\omega_{\infty}$  can be left out.

7. The Variational Formula Corresponding to Monogeneous Squilibrium Equations. We assume that X = M = 0 and consider the integral

Substituting in this in place of  $8^{\alpha\beta}$  and  $8^{\alpha\beta}$  their suppression in terms of stress functions (3.24) and (3.25) and integrating the result by parts, we get

$$I_{\sigma} = I_{c} - \iint \left\{ \psi_{\gamma} c_{\bullet}^{\alpha} \pi_{c_{\bullet}}^{\beta} \nabla_{\pi} + (a \cdot a \cdot b_{\alpha}) \right\}$$
 (7.2)

In order to transform the double integral in (7.2) we note that

(\*\*\* - \*\*\* -

louing that

 $(B^{r}_{C} \times \pi \nabla \nabla_{R} \nabla_{A} + \beta_{B} \cdot (B^{r}_{C} \times \pi \nabla_{B} \nabla_{A} + \lambda_{B} \nabla_{A} \nabla_{A} + \lambda_{B} \nabla_{A} \nabla_{A}$ 

Then the integral (7.2) on taking (7.4) and (7.5) into account ensures the form:

In order to eliminate  $\nabla_{\alpha} *_{\omega \beta} *$  from this, we consider the integral

$$\begin{cases}
\sum_{\alpha \in \mathcal{A}} e^{\alpha \pi} e^{\alpha \beta} e^{\gamma \psi} \nabla_{r} \cdot \nabla_{n} \cdot \nabla_{\beta} e^{\alpha \alpha} e^{\alpha \alpha} \\
\sum_{\alpha \in \mathcal{A}} e^{\alpha \pi} e^{\beta \gamma} e^{\gamma \gamma} e^{\gamma \gamma} e^{\gamma \alpha} e^{\alpha \beta} e^{\alpha \alpha} e^{\gamma \gamma} e^{\gamma} e^{$$

to account of the identities  $c_* \propto \pi \nabla_{\pi^{**} \times \beta} * * c_* \propto \pi^{*} \circ_{\pi \beta} * \omega_{\pi}^*$  it is equal to:

Swietituting this in (7.6), we have

or, introducing here instead of  $C_n^{\alpha \pi} C_n^{\alpha \pi} V_{\gamma} * V_{\pi^{*0}} \times B$  its value taken from the condition of continuity of deformation referred to the coordinate system of the deformed surface U:

formula (6.4) and the formula  $2(e_{\angle\beta} - e_{\angle\beta}) = (\nabla_{\angle} v)(\nabla_{\beta} v)$ , we have

To setisty the boundary conditions (6.24)-(6.27) from (6.9) we get

Prom this, integrating by parts we get

This relies it possible to represent the second integral to (7.9) in the form

Then increase of (7.9) we get

Thus, the functional R is transformed to the form

Here the quantities p and q can be expressed by means of  $\psi$  and  $\psi_{\infty}$  to the desired accuracy.

Consequently, in the equilibrium state the functional (7.11) has a stationary value when the static boundary conditions are satisfied with respect to  $\psi$  and  $\psi$ , and also the supplementary condition (6.22). From the stationary condition  $\delta R = 0$  there results a system of three differential equations for the functions  $\psi$  and  $\psi_{\infty}$  (conditions of continuity of deformations expressed in terms of  $\psi$  and  $\psi_{\infty}$ ) and also natural boundary conditions for the stress functions.

For small deformations (7.11) is simplified and results in the

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